

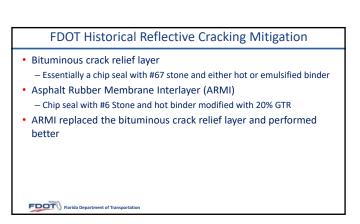
• 45,221 lane miles - 8,723 interstate lane miles (19%) - 34,074 arterial lane miles (76%) - 2,424 Turnpike lane miles (5%) • 97.2% of pavement is asphalt • 2.8% of pavement is concrete

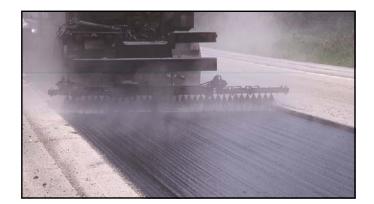
50.4% of asphalt surfaces are dense-graded (by lane mile) 49.6% of asphalt surfaces are open-graded Florida Department of Transportation

Asphalt Surfaces



Resurfacing • Sufficient budget not available to remove all the cracking on many projects • Resurfacing depths average around three inches • Crack depths are deeper in many cases









More Recently

- ARMI not being used today
- Mixed performance and opinions on effectiveness
- Difficult to construct
 - Lower production rates
 - Couldn't open to traffic until it was covered with mix
- Costly
- Performance was better with thicker mix coverage
- No structural coefficient
- Not recommended for urban or other low speed corridors
- Materials no longer readily available

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Replacement for the ARMI

- What's needed?
- A mix that can be produced with current materials
 - Must mitigate reflective cracking effectively
 - $-\operatorname{\mathsf{Can}}$ be opened to traffic
 - Has structural value
 - Easy to construct
 - Requires minimal cover
 - 1.5 2.0 inches
 - Has good rutting resistance

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Recent FDOT Research

- Evaluation of Reflective Cracking Mitigation Treatments Using the Composite Specimen Interface Cracking (CSIC) Test
 - FDOT contracted research with the University of Florida
 - Final report was issued in June 2018

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Project Objectives

- Develop guidelines for an effective alternative to the ARMI that was less expensive than a geotextile system
 - Identify key mixture characteristics that provide high fracture tolerance and shear resistance
 - Refine an existing test device to effectively evaluate reflective cracking performance
 - Develop preliminary mix design guidelines for mitigating reflective cracking on resurfacing projects

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Project Overview

- Research evaluated 14 mixtures covering a broad range of gradations.
 - The crack attenuating mixture (CAM) and binder rich intermediate course (BRIC) mixture were used as starting points
- Evaluated two granite aggregate sources and one limestone aggregate source commonly used in Florida
- Two binder types were considered: Polymer modified PG 76-22 and high polymer binder (HiMA)
- Two lift thicknesses were studied: 0.75" and 1.0"

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Gradations evaluated

- Six 9.5-mm dense-graded blends
 - 3 with limestone and 3 with granite
- Two 9.5-mm gap-graded
 - 1 with limestone and 1 with granite
- Six 4.75-mm dense-graded blends
 - $-\,3$ with limestone and 3 with granite

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Laboratory Analysis and Testing

- Mixtures designed using Superpave volumetrics and Dominant Aggregate Size Range – Interstitial Component (DASR-IC) analysis to optimize mixture gradations
 - Ndesign = 50 gyrations
 - Effective film thickness determined
- Composite Specimen Interface Cracking (CSIC) test developed by UF used to evaluate reflective cracking resistance
- Asphalt Pavement Analyzer (APA) used to evaluate mixture rutting

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Recent FDOT Research

Composite Specimen Interface Cracking (CSIC) Test

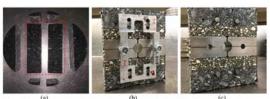


Figure 4-2. Assembly of CSIC composite specimen: (a) Cutting to obtain two symmetrical parts (plan view), (b) Two symmetrical parts aligned and glued to the central metal spacer, and (c) Complete CSIC specimen installed with gauge points.

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Recent FDOT Research • Composite Specimen Interface Cracking (CSIC) Test Cracked Specimen Cracked Specimen Cracked Specimen Figure 4-1. CSIC test setup: (a) Loading device, (b) Placement of CSIC specimen into the loading device, and (c) Councetion of loading device to the MTS loading frame.

Findings

- Gap-graded granite mixture provided superior reflective cracking performance
 - However, it would be significantly more costly (higher AC content and fibers) and not suitable for limestone aggregates
- Dense-graded 9.5-mm mixtures (for all aggregate types) with a minimum EFT of 35 μm provided good laboratory reflective cracking and rutting performance
 - More suitable overall than the 4.75-mm mixtures
- The DASR-IC model developed in this study provides a systematic approach for designing these mixtures with improved reflective cracking and rutting performance and reduced cost

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Recommended Future Work

- A broader range of aggregate types, gradations, and interlayer thicknesses should be studied to refine the design guidelines
- Development of a simpler system to design the mixtures
- HVS testing or a controlled test section should be performed to finalize and validate the guidelines and performance

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Current Research

- Follow up research project with UF to finalize mix design process and specification requirements
 - "Practical Mix Design Guidelines for Reflective Cracking Resistant Mixtures"
 - Expected completion in the spring 2023
- · Planned controlled test section on US 301 in Clay County
- Anticipated construction in the summer or fall 2023
- Planning HVS test sections for the next cycle of testing

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Current UF Research Project

- Project variables
 - Two optimized 9.5-mm dense gradations
 - 2.0% air voids at Ndesign = 50 gyrations
 - Two aggregate types (granite and limestone)
 - Three density levels (94%, 96%, and 98%)
- Laboratory performance tests
 - Rutting: APA and Hamburg
 - Cracking: CSIC and Texas Overlay
- Durability: Cantabro
- Two binders
 - Polymer modified PG 76-22 and high polymer binder (HiMA)
- · Consideration for using RAP

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US 301 Test Section

- Existing corridor with minimal pavement deficiencies
 - 21,000 AADT, 30% trucks
 - Mill existing pavement 3.75"
 - $-\,\mbox{Sawcut}$ longitudinal and transverse cracks to granular base
- Control Section
 - Place 3.00" dense-graded structural course with PG 76-22 binder
 - Place 0.75" open-graded friction course with PG 76-22 binder
- Test Section
 - Place 1.25" crack relief mix (HP binder)
 - Place 1.75" dense-graded structural course with PG 76-22 binder
 - Place 0.75" open-graded friction course with PG 76-22 binder

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